# HW 10, ME 440 Intermediate Vibration, Fall 2017 

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Problem 1. Use Newton's Law to determine the equation of motion. Solve for the natural frequencies and mode shapes without using a computer (solve by hand). Use your hand written solution to write out the $2 \times 2$ modal matrix (normalized) and the $2 \times 2 \Omega$ matrix.

Problem 2. Solve for the natural frequencies and mode shapes using Matlab. (Include a screen shot of your Matlab output.)

The sphere of mass $m$ is attached to the end of a cantilevered beam that is fixed to a carriage of mass $2 m$ as shown in the figure below. The generalized coordinates of the system are the absolute displacements $x_{1}$ and $x_{2}$ of the carriage and sphere, respectively. Determine (a) the mass and stiffness matrices of the system, and (b) the system's natural circular frequencies and modal matrix $[u]$ if $k=200 \mathrm{lb} / \mathrm{in}$. and $m=2 \mathrm{lb} \cdot \mathrm{s}^{2} / \mathrm{in}$.


Partial answer: $\omega_{2}=16.68 \mathrm{rad} / \mathrm{s}$

### 0.1 Problem 1

To make it easier to obtain the equation of motions, the top mass $m$ is modeled as attached to spring of stiffness $k$ which is in turn attached to an infinitely stiff vertical massless beam. This way the vibration of the mass $m$ at the top can be more easily modeled.


Simplified model of the original system

Based on the above diagram, we now obtain the free body diagram as follows. In this, we assume that $x_{2}>x_{1}$ and both as positive. Hence spring $k$ attached to $m$ is in tension.


The top mass $m$ vibrates in horizontal direction only. Hence this assumes the spring will remain horizontal and we must assume that $x_{2}-x_{1}$ remain small for this model to be realistic.

From this free body diagram we see now that the reaction force $F_{x}$ is equal to $k\left(x_{2}-x_{1}\right)$. (By resolving forces in the $x$ direction for the massless beam).

Therefore

$$
F_{x}=k\left(x_{2}-x_{1}\right)
$$

And the equation of motion for $x_{2}$ is

$$
\begin{align*}
m \ddot{x}_{2} & =-k\left(x_{2}-x_{1}\right) \\
m \ddot{x}_{2}+k x_{2}-k x_{1} & =0 \tag{1}
\end{align*}
$$

The equation of motion for the cart is

$$
\begin{align*}
2 m \ddot{x}_{1} & =-4 k x_{1}+F_{x} \\
2 m \ddot{x}_{1} & =-4 k x_{1}+k\left(x_{2}-x_{1}\right) \\
2 m \ddot{x}_{1}+5 k x_{1}-k x_{2} & =0 \tag{2}
\end{align*}
$$

Writing (1) and (2) in matrix form

$$
\left[\begin{array}{cc}
2 m & 0 \\
0 & m
\end{array}\right]\left\{\begin{array}{l}
\ddot{x}_{1} \\
\ddot{x}_{2}
\end{array}\right\}+\left[\begin{array}{cc}
5 k & -k \\
-k & k
\end{array}\right]\left\{\begin{array}{l}
x_{1} \\
x_{2}
\end{array}\right\}=\left\{\begin{array}{l}
0 \\
0
\end{array}\right\}
$$

Or

$$
\left[\begin{array}{ll}
4 & 0 \\
0 & 2
\end{array}\right]\left\{\begin{array}{l}
\ddot{x}_{1} \\
\ddot{x}_{2}
\end{array}\right\}+\left[\begin{array}{cc}
1000 & -200 \\
-200 & 200
\end{array}\right]\left\{\begin{array}{l}
x_{1} \\
x_{2}
\end{array}\right\}=\left\{\begin{array}{l}
0 \\
0
\end{array}\right\}
$$

The first step is to find the eigenvalues (which are the square of the natural frequency) for the system.

Let

$$
\begin{aligned}
A & =M^{-1} K \\
& =\left[\begin{array}{ll}
4 & 0 \\
0 & 2
\end{array}\right]^{-1}\left[\begin{array}{cc}
1000 & -200 \\
-200 & 200
\end{array}\right]
\end{aligned}
$$

But

$$
\begin{aligned}
{\left[\begin{array}{ll}
4 & 0 \\
0 & 2
\end{array}\right]^{-1} } & =\frac{1}{\operatorname{det}(M)}\left[\begin{array}{ll}
2 & 0 \\
0 & 4
\end{array}\right] \\
& =\frac{1}{8}\left[\begin{array}{ll}
2 & 0 \\
0 & 4
\end{array}\right] \\
& =\left[\begin{array}{ll}
\frac{1}{4} & 0 \\
0 & \frac{1}{2}
\end{array}\right]
\end{aligned}
$$

Hence

$$
\begin{aligned}
A & =\left[\begin{array}{ll}
\frac{1}{4} & 0 \\
0 & \frac{1}{2}
\end{array}\right]\left[\begin{array}{cc}
1000 & -200 \\
-200 & 200
\end{array}\right] \\
& =\left[\begin{array}{cc}
250 & -50 \\
-100 & 100
\end{array}\right]
\end{aligned}
$$

Now we will find the eigenvalues of $A$ (these will be the $\omega_{n}^{2}$ values). To find the eigenvalues of $A$, we solve

$$
\begin{aligned}
\operatorname{det}([A]-\lambda[I]) & =0 \\
\operatorname{det}\left(\left[\begin{array}{cc}
250 & -50 \\
-100 & 100
\end{array}\right]-\left[\begin{array}{cc}
\lambda & 0 \\
0 & \lambda
\end{array}\right]\right) & = \\
\left|\begin{array}{cc}
250-\lambda & -50 \\
-100 & 100-\lambda
\end{array}\right| & = \\
(250-\lambda)(100-\lambda)-5000 & =0 \\
\lambda^{2}-350 \lambda+20000 & =0
\end{aligned}
$$

Hence

$$
\begin{aligned}
\lambda & =\frac{-b}{2 a} \pm \frac{\sqrt{b^{2}-4 a c}}{2 a} \\
& =\frac{350}{2} \pm \frac{\sqrt{350^{2}-4(20000)}}{2} \\
& =175 \pm 103.08 \\
& =\{71.92,278.08\}
\end{aligned}
$$

Therefore, the eigenvalues are

$$
\begin{equation*}
\lambda=\omega_{n}^{2}=\{71.92,278.08\} \tag{3}
\end{equation*}
$$

The natural frequencies of the system are the sqrt of the eigenvalues. Therefore

$$
\begin{aligned}
\omega_{n} & =\{\sqrt{71.92}, \sqrt{278.08}\} \\
& =\{8.4806,16.676\}
\end{aligned}
$$

Hence

$$
\begin{aligned}
& \omega_{n(1)}=8.4806 \mathrm{rad} / \mathrm{sec} \\
& \omega_{n(2)}=16.676 \mathrm{rad} / \mathrm{sec}
\end{aligned}
$$

The next step is to find the eigenvectors. These are also called the shape vectors, or the $u$ vectors. Each eigenvalue will generate one eigenvector. We need to solve

$$
[A]\{u\}=\lambda\{u\}
$$

For each eigenvalue, we find the corresponding eigenvector.
For $\lambda=71.92$, we obtain the equation

$$
\left[\begin{array}{cc}
250 & -50 \\
-100 & 100
\end{array}\right]\left\{\begin{array}{l}
u_{11} \\
u_{21}
\end{array}\right\}=71.92\left\{\begin{array}{l}
u_{11} \\
u_{21}
\end{array}\right\}
$$

From first equation

$$
250 u_{11}-50 u_{21}=71.92 u_{11}
$$

We always let $u_{11}=1$. Therefore

$$
\begin{aligned}
250-50 u_{21} & =71.92 \\
u_{21} & =\frac{250-71.92}{50} \\
& =3.5616
\end{aligned}
$$

Therefore, the first eigenvector is

$$
\vec{u}_{1}=\left\{\begin{array}{c}
1 \\
3.5616
\end{array}\right\}
$$

For $\lambda=278.08$, we obtain the equation

$$
\left[\begin{array}{cc}
250 & -50 \\
-100 & 100
\end{array}\right]\left\{\begin{array}{l}
u_{12} \\
u_{22}
\end{array}\right\}=278.08\left\{\begin{array}{l}
u_{12} \\
u_{22}
\end{array}\right\}
$$

From first equation

$$
250 u_{12}-50 u_{22}=278.08 u_{12}
$$

We always let $u_{12}=1$. Hence

$$
\begin{aligned}
250-50 u_{22} & =278.08 \\
u_{22} & =\frac{250-278.08}{50} \\
& =-0.5616
\end{aligned}
$$

Therefore, the second eigenvector is

$$
\vec{u}_{2}=\left\{\begin{array}{c}
1 \\
-0.5616
\end{array}\right\}
$$

Therefore the modal matrix [ $u$ ] is

$$
u=\left[\begin{array}{cc}
1 & 1 \\
3.5616 & -0.5616
\end{array}\right]
$$

And $\Omega$ matrix is

$$
\begin{aligned}
\Omega & =\left[\begin{array}{cc}
\omega_{n(1)}^{2} & 0 \\
0 & \omega_{n(2)}^{2}
\end{array}\right] \\
& =\left[\begin{array}{cc}
71.92 & 0 \\
0 & 278.08
\end{array}\right]
\end{aligned}
$$

And the system of equations written in principle coordinates $q$ is

$$
\begin{array}{r}
\{\ddot{q}\}+[\Omega]\{q\}=\{0\} \\
{\left[\begin{array}{ll}
1 & 0 \\
0 & 1
\end{array}\right]\left\{\begin{array}{l}
\ddot{q}_{1}(t) \\
\ddot{q}_{2}(t)
\end{array}\right\}+\left[\begin{array}{cc}
71.92 & 0 \\
0 & 278.08
\end{array}\right]\left\{\begin{array}{l}
\ddot{q}_{1}(t) \\
\ddot{q}_{2}(t)
\end{array}\right\}=\left\{\begin{array}{l}
0 \\
0
\end{array}\right\}}
\end{array}
$$

which is now decoupled. The solution in normal coordinates is

$$
\begin{aligned}
\left\{\begin{array}{l}
x_{1}(t) \\
x_{2}(t)
\end{array}\right\} & =A_{1}\left\{\begin{array}{l}
u_{11} \\
u_{21}
\end{array}\right\} \cos \left(\omega_{n(1)} t-\phi_{1}\right)+A_{2}\left\{\begin{array}{l}
u_{12} \\
u_{22}
\end{array}\right\} \cos \left(\omega_{n(2)} t-\phi_{2}\right) \\
& =A_{1}\left\{\begin{array}{c}
1 \\
3.5616
\end{array}\right\} \cos \left(8.481 t-\phi_{1}\right)+A_{2}\left\{\begin{array}{c}
1 \\
-0.5616
\end{array}\right\} \cos \left(16.676 t-\phi_{2}\right)
\end{aligned}
$$

### 0.1.1 Appendix

This is derivation of the same equations of motions using energy method. (In this example, this method is much simpler to use to find equation of motions). The kinetic energy of the system is

$$
T=\frac{1}{2} m \dot{x}_{2}^{2}+\frac{1}{2}(2 m) \dot{x}_{1}^{2}
$$

And the potential energy comes only from the springs, since we assumed the top mass $m$ remain horizontal as it vibrates back and forth

$$
U=\frac{1}{2} 4 k x_{1}^{2}+\frac{1}{2} k\left(x_{2}-x_{1}\right)^{2}
$$

Therefore the Lagrangian is

$$
\begin{aligned}
\Gamma & =T-U \\
& =\frac{1}{2} m \dot{x}_{2}^{2}+m \dot{x}_{1}^{2}-\frac{1}{2}(4 k) x_{1}^{2}-\frac{1}{2} k\left(x_{2}-x_{1}\right)^{2}
\end{aligned}
$$

$\underline{\text { EQM for } x_{1}}$

$$
\begin{align*}
\frac{d}{d t}\left(\frac{\partial \Gamma}{\dot{x}_{1}}\right)-\frac{\partial \Gamma}{x_{1}} & =0 \\
\frac{d}{d t}\left(2 m \dot{x}_{1}\right)-\left(-4 k x_{1}+k\left(x_{2}-x_{1}\right)\right) & =0 \\
2 m \ddot{x}_{1}-\left(-4 k x_{1}+k x_{2}-k x_{1}\right) & =0 \\
2 m \ddot{x}_{1}-\left(-5 k x_{1}+k x_{2}\right) & =0 \\
2 m \ddot{x}_{1}+5 k x_{1}-k x_{2} & =0 \tag{1}
\end{align*}
$$

$\underline{\text { EQM for } x_{2}}$

$$
\begin{align*}
\frac{d}{d t}\left(\frac{\partial \Gamma}{\dot{x}_{2}}\right)-\frac{\partial \Gamma}{x_{2}} & =0 \\
\frac{d}{d t}\left(m \dot{x}_{2}\right)-\left(-k\left(x_{2}-x_{1}\right)\right) & =0 \\
m \ddot{x}_{2}-\left(-k x_{2}+k x_{1}\right) & =0 \\
m \ddot{x}_{2}+k x_{2}-k x_{1} & =0 \tag{2}
\end{align*}
$$

In Matrix form $(1,2)$ becomes

$$
\left[\begin{array}{cc}
2 m & 0 \\
0 & m
\end{array}\right]\left\{\begin{array}{l}
\ddot{x}_{1} \\
\ddot{x}_{2}
\end{array}\right\}+\left[\begin{array}{cc}
5 k & -k \\
-k & k
\end{array}\right]\left\{\begin{array}{l}
x_{1} \\
x_{2}
\end{array}\right\}=\left\{\begin{array}{l}
0 \\
0
\end{array}\right\}
$$

Which is the same exact result obtained earlier.

### 0.2 Problem 2

The Matlab code is the following

```
%Solve HW 10, problem 2 using Matlab
%Nasser M. Abbasi, ME 440, Fall 2017
%see HW 10 for more details
m = 2;
k = 200;
mass_mat = [2*m 0;
    m]
stiffness_mat = [5*k -k;
    -k k]
A_mat = inv(mass_mat) * stiffness_mat
[eig_vectors, eig_values] = eig(A_mat);
natural_frequencies = sqrt(diag( eig_values))
eig_vectors(:,1) = eig_vectors(:,1)/eig_vectors(1,1);
eig_vectors(:,2) = eig_vectors(:,2)/eig_vectors(1,2);
eig_vectors
```

The output is

```
mass_mat =
    4 0
    0
```

stiffness_mat =
1000 -200
-200 200
A_mat =
$250-50$
-100 100
natural_frequencies =
16.6757
8.4807
eig_vectors =
$1.0000 \quad 1.0000$
$-0.5616 \quad 3.5616$

### 0.3 Problem 3

## Problem 3.

Determine the flexibility matrix of the uniform beam shown in the figure below. Disregard the mass of the beam compared to the concentrated masses fastened on the beam and assume the beam has a stiffness of $E I$ and that all $l_{i}=l$.


Definitions For stiffness matrix [ $K$ ], element $k_{i j}$ means: Apply unit displacement at location $j$ and measure the force at location $i$. While for flexibility matrix [ $a$ ], its element $a_{i j}$ means: Apply unit force at location $j$ and measure the displacement at location $i$.

To solve this problem, this part of handout is used
Fixed-fixed beam*

(i)

$$
\begin{array}{ll}
y=\frac{P b^{2}}{6 E I l^{3}}\left[(2 b-3 l) x^{3}+3 l(l-b) x^{2}\right] & (x \leq a) \\
y=\frac{P b^{2}}{6 E I l^{3}}\left[(2 b-3 l) x^{3}+3 l(l-b) x^{2}+\frac{l^{3}}{b^{2}}(x-a)^{3}\right] & (x \geq a)
\end{array}
$$

Since [a] is symmetric, only lower triangle part needs to be found (or upper triangle).

$$
\left[\begin{array}{lll}
a_{11} & & \\
a_{21} & a_{22} & \\
a_{31} & a_{32} & a_{33}
\end{array}\right]
$$

To find $a_{11}$, a unit force is put at location $m_{1}$ and displacement at $m_{1}$ is measured. To find $a_{21}$, a unit force is put at location $m_{1}$ and displacement at $m_{2}$ is measured and so on. The formulas in the above hand out are used for this. To speed this process and make less error, a small function is written to do the computation. Here is the function and the result generated for $a_{11}, a_{21}, a_{32}, a_{22}, a_{32}, a_{33}$

## Define the function to find a_ij

getFlexibility $\left[x_{-}, a_{-}, b_{-}\right]:=$Piecewise $[\{$

$$
\begin{aligned}
& \left\{\frac{b^{2}}{6 \mathrm{E} 0 \mathrm{I} 0 \mathrm{~L} \theta^{3}}\left((2 b-3 \mathrm{~L} \theta) x^{3}+3 \mathrm{~L} 0(\mathrm{~L} \theta-b) x^{2}\right), x \leq a\right\} \\
& \left.\left.\left\{\frac{b^{2}}{6 \mathrm{E} 0 \mathrm{I} 0 \mathrm{~L} \theta^{3}}\left((2 b-3 \mathrm{~L} 0) x^{3}+3 \mathrm{~L} 0(\mathrm{~L} \theta-b) x^{2}+\frac{\mathrm{L} \theta^{3}}{b^{2}}(x-a)^{3}\right), x>a\right\}\right\}\right]
\end{aligned}
$$

Call the function to find each element in lower triangle
$\ln [43]=\mathbf{L} \boldsymbol{0}=\mathbf{4 L}$;
$a=L ; b=3 L ; x=L ;$
flex[1, 1] = Assuming[x > 0, Simplify[getFlexibility[x, a, b]]]
Out[45]= $=\frac{9 L^{3}}{64 E 0 \text { I0 }}$
$\ln [48]:=\mathrm{a}=\mathrm{L} ; \mathrm{b}=3 \mathrm{~L} ; \mathbf{x}=\mathbf{2 L}$;
flex[2, 1] = Assuming[x > 0, Simplify[getFlexibility[x, a, b]]]
Out[49]= $\frac{L^{3}}{6 E 0 ~ I 0}$
$\ln [50]:=a=L ; b=3 L ; x=3 L ;$
flex[3, 1] = Assuming[x>0, Simplify[getFlexibility[x, a, b]]]
Out[51] $=\frac{13 L^{3}}{192 \mathrm{E} 0 \mathrm{I} 0}$
$\ln [52]=\mathbf{a}=\mathbf{2 L ;} \mathbf{b}=\mathbf{2 L ;} \mathbf{x}=\mathbf{2 L}$;
flex[2, 2] = Assuming[x>0, Simplify[getFlexibility[x, a, b]]]
Out[53]= $\frac{L^{3}}{3 E 0 ~ I 0}$
$\ln [54]=\mathbf{a}=\mathbf{2 L ;} \mathbf{b}=\mathbf{2 L} \mathbf{L} \mathbf{x}=\mathbf{3 L}$;
flex[3, 2] = Assuming[x>0, Simplify[getFlexibility[x, a, b]]]
Out[55] $=\frac{L^{3}}{6 \mathrm{E} 0 \text { I0 }}$
$\ln [56]:=\mathbf{a}=\mathbf{3 L} \mathbf{~} \mathbf{b}=\mathrm{L} ; \mathbf{x}=\mathbf{3 L}$;
flex[3, 3] = Assuming[x > 0, Simplify[getFlexibility[x, a, b]]]
Out $[57]=\frac{9 L^{3}}{64 E 0 \text { I0 }}$

Therefore, using this result, the lower triangle is

$$
\left[\begin{array}{ccc}
\frac{9}{64} & & \\
\frac{1}{6} & \frac{1}{3} & \\
\frac{13}{192} & \frac{1}{6} & \frac{9}{64}
\end{array}\right] \frac{L^{3}}{E I}
$$

Hence by symmetry

$$
[a]=\left[\begin{array}{ccc}
\frac{9}{64} & \frac{1}{6} & \frac{13}{192} \\
\frac{1}{6} & \frac{1}{3} & \frac{1}{6} \\
\frac{13}{192} & \frac{1}{6} & \frac{9}{64}
\end{array}\right] \frac{L^{3}}{E I}
$$

